



July 22, 2025 (Rev.2)

Our File: 24-9848

Via email: tim@paragonhomesltd.com

Paragon Homes Ltd.
14205 Malabar Avenue
White Rock, BC, V4B 2Y4

Attn: Tim Ratzlaff

Re: Preliminary Geotechnical Exploration Report
Proposed Subdivision
14205 Malabar Avenue, White Rock, BC

***Foundations,
Excavation &
Shoring
Specialists***

Braun Geotechnical
102 – 19040 95A Ave.
Surrey, BC
V4N 4P3
Tel: 604-513-4190

1.0 INTRODUCTION

As requested, Braun Geotechnical Ltd. (BGL) carried out a geotechnical exploration for the above-referenced project. The geotechnical work has been performed in general accordance with the terms and conditions of the BGL proposal dated May 8, 2024 (our reference no. P24-8524). The scope of work included onsite subsurface exploration and provision of preliminary geotechnical report for the proposed subdivision.

The scope of services was limited to the evaluation of geotechnical characteristics at the site. No consideration was given to any environmental aspects.

2.0 SITE DESCRIPTION AND PROPOSED DEVELOPMENT

The subject site was located at 14205 Malabar Avenue, in the City of White Rock, BC. The subject property is approximately rectangular in plan, with maximum overall plan dimensions of approximately 27 m by 38 m. The subject property gently slopes to the south with existing grades ranging from approximately El. 71.5 m to El. 73.5 m.

Based on the City of White Rock Online Mapping System, the southeast corner of the property is included in the Environmental Development Permit Area. This should be reviewed by a qualified Environmental Engineer.

It is understood that the property may be subdivided into two lots for Single Family Dwellings (SFDs) with one level of basement.

At the time of geotechnical exploration, the site was occupied by an existing SFD with associated driveway, garage and landscape areas.

3.0 SITE EXPLORATION

Two test pits (TP24-01 and TP24-02) were excavated on May 18, 2024, using a tracked excavator to depths of 1.8 m and 1.5 m respectively. The location and number of test pits that could be excavated was limited due to the site access constraints.

The soil conditions were logged in the field by BGL, and representative soil samples were returned to our laboratory for further visual classification and water content

Foundations

***Excavation &
Shoring***

Slope Stability

Natural Hazards

***Pavement Design
and Management***

***Reinforced Soil
Walls and Slopes***

PTP # 1002594

testing. Test pit locations are shown on the attached Location Plan (Dwg. no. 24-9848-01).

4.0 SOIL AND GROUNDWATER CONDITIONS

A review of available published and in-house geological information indicated that the study site area is underlain by Capilano Sediments, including marine, glaciomarine stony (including till-like deposits) to stoneless silt loam to clay loam, with minor sand and silt normally less than 3 m thick, but up to 30 m thick.

The findings of the test pit exploration are detailed on the attached test pit logs. A generalized subsoil profile based on the test pit logs is summarized below.

FILL/ORGANICS

Dark-brown, damp, loose silty SAND to firm sandy SILT with some organics and occasional roots/rootlets was encountered within TH24-01 and TH24-02 to depths of approximately 0.3 m and 0.2 m, respectively.

SAND

Rust-brown, damp, compact SAND with some silt, a trace of gravel and occasional roots/rootlets was encountered within all test pits to a depth of approximately 0.50 m.

Sandy SILT

Brown, occasionally rust-mottled, damp, stiff fine sandy SILT was encountered below within TH24-02 to a depth of 0.75 m.

SAND to Silty SAND

Brown, damp, dense to very dense SAND with some silt to silty SAND with a trace to some gravel was encountered within each of the test pits to the depth of test pit exploration at 1.5 m to 1.8 m. The full thickness of this unit was not investigated.

GROUNDWATER

Groundwater/Seepage was not encountered within the test pits at the time of exploration. Depending on seasonal conditions, near-surface seepage flows should be anticipated within natural and fill soil layers overlying the relatively low permeable dense to very dense soils and/or within sand seams. Groundwater levels and near-surface run-off flows are expected to fluctuate seasonally, and with drainage conditions.

The subsurface conditions described above were encountered at the test pit locations only. Subsurface conditions at other locations could vary.

5.0 DISCUSSION AND RECOMMENDATIONS

5.1 General

The test pits generally encountered approximately 0.2 m to 0.3 m thickness of fill/organics over compact sand to a depth of approximately 0.5 m underlain by stiff fine sandy silt to a depth of approximately 0.75 m. This was underlain by dense to very dense sand with some silt to silty sand soils to the maximum depths of test pit exploration.

The foundations for the proposed SFDs could be supported on the underlying natural dense to very dense soils, and/or on structural fills placed thereon, using conventional shallow strip and pad footings.

The following sections provide our geotechnical recommendations for site preparation and foundation design.

5.2 Foundation Excavation

Based on the information provided by the Client, the subdivided lots will be developed into single-family dwellings (SFDs) each with one level of basement.

Temporary unsupported excavation slopes may be feasible where space permits, or suitable support systems (shoring) should be provided. It is anticipated that the excavation could be kept free of any standing water using localized sumps and pumping.

It is anticipated that unsupported excavations could be sloped 1H:1V (horizontal to vertical) in fill and overburden/organic-rich materials, at 3H:4V in compact sand, at 2H:3V in stiff silt and dense to very dense soils. Flatter slopes may be required if poor soil conditions or seepage is encountered. Temporary excavation slopes should be reviewed and approved by BGL at the time of construction.

All excavation slopes should be covered with polyethylene sheeting held securely in place at the crest and toe of slopes, for moisture control and erosion protection.

Where unsupported excavation slopes are not feasible (geometrically), temporary shoring would be required. We note that shotcrete and anchor shoring or sloped excavations extending into any adjacent properties would require encroachment agreements with the adjoining property owners. These agreements should be obtained prior to the start of construction. If requested, excavation shoring drawings would be provided under a separate cover.

From our experience on nearby sites, cobbles and boulders may be encountered during site preparation activities. Boulders could require additional excavation measures such as blasting, breaking or splitting.

5.3 Site Preparation

Site preparation below the proposed structures, pavement, and site fill areas, should include removal of all vegetation, organic soils, soft disturbed soils, existing fill and other deleterious material down to natural, undisturbed soils free of organics.

The natural upper stiff fine sandy silt, free of significant organics is considered suitable subgrade for support of pavements. Stripping for proposed residential structures should extend to the underlying dense to very dense sand soils.

Drainage measures should be implemented to reduce potential for water ponding on exposed subgrades. Temporary and final grades should be established to avoid uncontrolled offsite discharge of surface and/or near-surface run-off flows.

Stripped surfaces should be reviewed by BGL prior to placing foundations or structural fills.

Large boulders may be encountered during site preparation activities. Large boulders encountered during site excavation could require additional excavation measures such as blasting or rock splitting.

5.4 Structural Fill & Trench Backfill

Subgrade restoration fills & general trench backfills below roadway areas should consist of structural fill comprised of Master Municipal Construction Documents (MMCD) compliant Select Granular Subbase. Structural fill should be placed and compacted in maximum 300 mm loose layers with each layer compacted to at least 95% of Modified Proctor Maximum Dry Density (MPMDD). For confined areas, structural fill placed under building and asphalt pavements should extend horizontally beyond by a distance equal to at least the thickness of structural fill layer. Unconfined fills should typically extend horizontally by a distance equal to 2 times the thickness of structural fill layer.

Density testing should be carried out during fill placement on a regular basis to confirm adequacy of compaction, and the results forwarded to BGL for review. BGL should also be contacted to review fill quality, and placement and compaction procedures.

5.5 Slopes

5.5.1 Temporary Cut Slopes and Utility Trenches

Temporary excavations for worker entry may be slope cut, or alternatively suitable support systems should be provided. It is anticipated that proposed utility excavations could be achieved using conventional excavation and/or trench box methods. Additionally, excavations could be kept free of standing water using localized sumps and pumping.

For preliminary planning purposes, unsupported excavation cuts should be sloped at 1H:1V in fill and overburden/organic-rich materials, at 3H:4V in compact sand, at 2H:3V in stiff silt and dense to very dense soils (6 m max). These recommended slope geometries should be reviewed by BGL during excavation and may require modification based on actual site conditions. Flatter slopes may be required if poor soil conditions or significant seepage is encountered.

5.5.2 Permanent Slopes

The recommended maximum permanent cut slope geometry is 2H:1V. Fill slopes consisting of suitably compacted native mineral or import granular soils should be constructed at gradients no steeper than 2.5H:1V. Permanent slopes should typically be planted or otherwise protected from erosion as soon as practical.

Any deterioration of slopes should be immediately reported to the BGL. Based on BGL's review, recommendations for slope stabilization would be provided which may include flattening of the slopes in addition to other possible mitigative measures.

5.6 Residential House Foundations

It is recommended that foundations for the proposed SFDs be supported on natural dense to very dense soils, and/or structural fills placed thereon. Basement levels would be feasible for geotechnical considerations.

The following soil resistance (bearing) values may be adopted for preliminary foundation design:

Foundation Subgrade	Limit States Design		Working Stress Design
	Factored Ultimate Bearing Resistance (ULS) ¹	Serviceability Limit State (SLS)	Allowable Bearing Pressure DL + LL
Natural Dense to Very Dense Soils or Compacted Structural Fill	150 kPa (3100 psf)	100 kPa (2050 psf)	100 kPa (2050 psf)

Note: 1. The factored ultimate bearing resistance values include a geotechnical resistance factor of 0.5.
2. Higher bearing values may be feasible for specific situations, subject to review by BGL

The above design bearing pressures for soil subgrade assume the following:

- Strip and pad footings have minimum widths of 450 mm (18") and 600 mm (24"), respectively.
- Footings are founded at least 450 mm (18") below final finished adjacent grade.

- Site preparation is completed as indicated above and load-bearing surfaces are reviewed and approved by BGL.
- Foundation bearing surfaces are no higher than 2H:1V from the base or toe of adjacent walls, retaining structures, etc.
- Footings are placed below a 1H:1V line projected up from lower footings or buried structures such as utility lines, sumps, etc.
- Silty subgrade areas are protected immediately after exposure.

Any soft, wet, or deleterious material encountered at bearing surface level should be sub-excavated and replaced with structural fill. The structural fill should be placed and compacted in maximum 300 mm thick layers with each layer compacted to at least 95% of MPMDD.

5.7 Backfill

Perimeter backfill and fill for support of exterior residential sidewalks, driveway, patios, etc. should typically consist of relatively clean, well-graded, granular material (reviewed and approved by BGL). The fill should be placed and compacted in maximum 300 mm thick loose layers to at least 95% of MPMDD.

Walk behind plate tamper compactors should be used to compact backfill within 1 m of foundation walls to avoid excessive buildup of lateral earth stresses against the walls and the fill thickness in these areas should typically be reduced to 200 mm.

All backfill should be placed in a manner that avoids damaging the foundation walls, perimeter drains, and damp-proofing or waterproofing on the wall. Proposed grades should slope away from the proposed structures to promote flow of surface water runoff away from the structures. A 300 mm thick layer of relatively impermeable soil should be placed at surface to minimize surface water entering the perimeter fill and, in turn, the perimeter drainage system.

5.8 Slab on Grade

The slab on grade should be underlain by a drainage layer comprising a minimum 100 mm (4") thick layer of 20 mm clear crushed gravel (MMCD Coarse Drain Rock). This drainage layer should have a suitable connection to the permanent storm sewer system. Polyethylene sheeting should also be provided beneath the floor slab to further reduce potential slab dampness.

Compaction testing should be carried out on underslab fills to confirm that all fill placed below the building has been compacted to at least 95% of MPMDD. Prior to placement of any grade restoration fills, the subgrade should be reviewed by BGL.

Assessment of radon gas should be completed by a consultant specializing in this area/field. If required, radon gas mitigation measures, as designed by the mechanical engineer, should be incorporated into the underslab preparation (refer to BCBC 2024).

5.9 Perimeter Drainage

Perimeter drainage should consist of perforated PVC pipe, placed around the building perimeters, with the invert elevation at footing level. The PVC pipe should be sized accordingly by the mechanical engineer/consultant. The perimeter drain pipe should be surrounded by at least 150 mm (6") of 20 mm (¾") clear crushed gravel (MMCD Coarse Drain Rock). A 150 mm (6") thick layer of Birdseye gravel should be placed over the clear crushed gravel to act as a filter layer.

5.10 Seismic Considerations

The 2024 BC Building Code (BCBC) based on NBCC 2020 6th Generation model classifies a site as X_c (formerly “Site Class C” – NBCC2015) where the subgrade soils in the upper 30 m consist of “very dense soil and soft rock” with average shear wave velocity (V_{S30}) between 360 m/sec and 760 m/sec, average SPT N values greater than 50 blows/300 mm and average undrained shear strength (S_u) greater than 100 kPa. Based on the test hole logs and mapping by the Geological Survey of Canada, very dense soils are present below a relatively shallow depth, corresponding to X_c.

The subgrade soil conditions are not considered susceptible to seismically induced liquefaction for the 2020 NBCC ground motions with a 2% probability of exceedance in 50 years (~1:2475 return).

5.11 Lateral Earth Pressures

For preliminary considerations, a uniform lateral pressure of 20 kPa (400 psf) is recommended for both static (including compaction induced stress) and static + seismic conditions for the design of walls 3.0 m (10 feet) or less in height provided that the backfill behind the wall is horizontal and fully drained.

6.0 GEOTECHNICAL FIELD REVIEWS

Geotechnical field reviews are required by the Geotechnical Registrant and to satisfy the requirements of the Letters of Professional Assurance required for the Building Permit. Field reviews are essential to confirm that the recommendations of the geotechnical report are understood and followed.

Geotechnical field reviews should be arranged by the Contractor to address the following:

- Removal of unsuitable materials below building footprint areas;
- Review of temporary excavation slopes;
- Suitability of exposed footing subgrade;
- Review and density testing of structural fill placed below building footings and slabs, and
- Review of perimeter drain installation with respect to geotechnical considerations only (if required).

7.0 APPLICABLE LEGISLATION

This geotechnical report may be considered a Landslide Assessment Report (EGBC Appendix D: Landslide Assessment Assurance Statement) to confirm that the “site may be used safely for the use intended” under Section 56 of the Community Charter. Safe site use is defined as two residential lot subdivision development in accordance with the current British Columbia Building Code. The subject site and proposed development are considered to be stable for global stability considerations for static and design seismic loading conditions.

8.0 CLOSURE

This report should be considered preliminary and is subject to review and revision as required once Civil, Architectural and Structural design details have been finalized.

This report is prepared for the exclusive use of Paragon Homes Ltd. and their designated representatives and may not be used by other parties without the written permission of BGL. The City of White Rock may also rely on the findings of this report.

If the development plans change, or if during construction soil conditions are noted to be different from those described in this report, BGL should be notified immediately in order that the geotechnical recommendations can be confirmed or modified, as required. Further, this report assumes that field reviews would be completed by BGL during construction.

The site Contractor should make their own assessment of subsurface conditions and select the construction means and methods most appropriate to the site conditions.

This report should not be included in the specifications without suitable qualifications approved by BGL.

The use of this assessment report is subject to the conditions on the attached Report Interpretation and Limitations sheet. The reader's attention is drawn specifically to those conditions, as it is considered essential that they be followed for proper use and interpretation of this report.

We hope the above meets your requirements. Should any questions arise, please do not hesitate to contact the undersigned.

Yours truly,

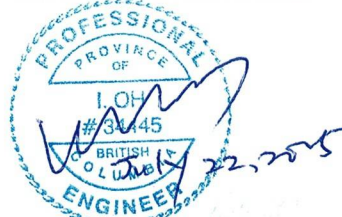
Braun Geotechnical Ltd.



2025-07-22

Avninder Singh Cheema, EIT.
Geotechnical Engineer

Braun Geotechnical Ltd.



Joseph Oh, P.Eng.
Geotechnical Engineer

Independently Reviewed by:



2025-07-22

Gunther Yip, P.Eng.
Geotechnical Engineer

Enclosures: Report Interpretation and Limitations
Location Plan
Test Pit Logs (2)

REPORT INTERPRETATION AND LIMITATIONS

1. STANDARD OF CARE

Braun Geotechnical Ltd. (Braun) has prepared this report in a manner consistent with generally accepted engineering consulting practices in this area, subject to the time and physical constraints applicable. No other warranty, expressed or implied, is made.

2. COMPLETENESS OF THIS REPORT

This Report represents a summary of paper, electronic and other documents, records, data and files and is not intended to stand alone without reference to the instructions given to Braun by the Client, communications between Braun and the Client, and/or to any other reports, writings, proposals or documents prepared by Braun for the Client relating to the specific site described herein.

This report is intended to be used and quoted in its entirety. Any references to this report must include the whole of the report and any appendices or supporting material. Braun cannot be responsible for use by any party of portions of this report without reference to the entire report.

3. BASIS OF THIS REPORT

This report has been prepared for the specific site, development, design objective, and purpose described to Braun by the Client or the Client's Representatives or Consultants. The applicability and reliability of any of the factual data, findings, recommendations or opinions expressed in this document pertain to a specific project as described in this report and are not applicable to any other project or site, and are valid only to the extent that there has been no material alteration to or variation from any of the descriptions provided to Braun. Braun cannot be responsible for use of this report, or portions thereof, unless we were specifically requested by the Client to review and revise the Report in light of any alterations or variations to the project description provided by the Client.

If the project does not commence within 18 months of the report date, the report may become invalid and further review may be required.

The recommendations of this report should only be used for design. The extent of exploration including number of test pits or test holes necessary to thoroughly investigate the site for conditions that may affect construction costs will generally be greater than that required for design purposes. Contractors should rely upon their own explorations and interpretation of the factual data provided for costing purposes, equipment requirements, construction techniques, or to establish project schedule.

The information provided in this report is based on limited exploration, for a specific project scope. Braun cannot accept responsibility for independent conclusions, interpretations, interpolations or decisions by the Client or others based on information contained in this Report. This restriction of liability includes decisions made to purchase or sell land.

4. USE OF THIS REPORT

The contents of this report, including plans, data, drawings and all other documents including electronic and hard copies remain the copyright property of Braun. However, we will consider any reasonable request by the Client to approve the use of this report by other parties as "Approved Users." With regard to the duplication and distribution of this Report or its contents, we authorize only the Client and Approved Users to make copies of the Report only in such quantities as are reasonably necessary for the use of this Report by those parties. The Client and "Approved Users" may not give, lend, sell or otherwise make this Report or any portion thereof available to any other party without express written permission from Braun. Any use which a third party makes of this Report – in its entirety or portions thereof – is the sole responsibility of such third parties. BRAUN GEOTECHNICAL LTD. ACCEPTS NO RESPONSIBILITY FOR DAMAGES SUFFERED BY ANY PARTY RESULTING FROM THE UNAUTHORIZED USE OF THIS REPORT.

Electronic media is susceptible to unauthorized modification or unintended alteration, and the Client should not rely on electronic versions of reports or other documents. All documents should be obtained directly from Braun.

5. INTERPRETATION OF THIS REPORT

Classification and identification of soils and rock and other geological units, including groundwater conditions have been based on exploration(s) performed in accordance with the standards set out in Paragraph 1. These tasks are judgemental in nature; despite comprehensive sampling and testing programs properly performed by experienced personnel with the appropriate equipment, some conditions may elude detection. As such, all explorations involve an inherent risk that some conditions will not be detected.

Further, all documents or records summarizing such exploration will be based on assumptions of what exists between the actual points sampled at the time of the site exploration. Actual conditions may vary significantly between the points investigated and all persons making use of such documents or records should be aware of and accept this risk.

The Client and “Approved Users” accept that subsurface conditions may change with time and this report only represents the soil conditions encountered at the time of exploration and/or review. Soil and ground water conditions may change due to construction activity on the site or on adjacent sites, and also from other causes, including climactic conditions.

The exploration and review provided in this report were for geotechnical purposes only. Environmental aspects of soil and groundwater have not been included in the exploration or review, or addressed in any other way.

The exploration and Report is based on information provided by the Client or the Client's Consultants, and conditions observed at the time of our site reconnaissance or exploration. Braun has relied in good faith upon all information provided. Accordingly, Braun cannot accept responsibility for inaccuracies, misstatements, omissions, or deficiencies in this Report resulting from misstatements, omissions, misrepresentations or fraudulent acts of persons or sources providing this information.

6. DESIGN AND CONSTRUCTION REVIEW

This report assumes that Braun will be retained to work and coordinate design and construction with other Design Professionals and the Contractor. Further, it is assumed that Braun will be retained to provide field reviews during construction to confirm adherence to building code guidelines and generally accepted engineering practices, and the recommendations provided in this report. Field services recommended for the project represent the minimum necessary to confirm that the work is being carried out in general conformance with Braun's recommendations and generally accepted engineering standards. It is the Client's or the Client's Contractor's responsibility to provide timely notice to Braun to carry out site reviews. The Client acknowledges that unsatisfactory or unsafe conditions may be missed by intermittent site reviews by Braun. Accordingly, it is the Client's or Client's Contractor's responsibility to inform Braun of any such conditions.

Work that is covered prior to review by Braun may have to be re-exposed at considerable cost to the Client. Review of all Geotechnical aspects of the project are required for submittal of unconditional Letters of Assurance to regulatory authorities. The site reviews are not carried out for the benefit of the Contractor(s) and therefore do not in any way effect the Contractor(s) obligations to perform under the terms of his/her Contract.

7. SAMPLE DISPOSAL

Braun will dispose of all samples 1 month after issuance of this report, or after a longer period of time at the Client's expense if requested by the Client. All contaminated samples remain the property of the Client and it will be the Client's responsibility to dispose of them properly.

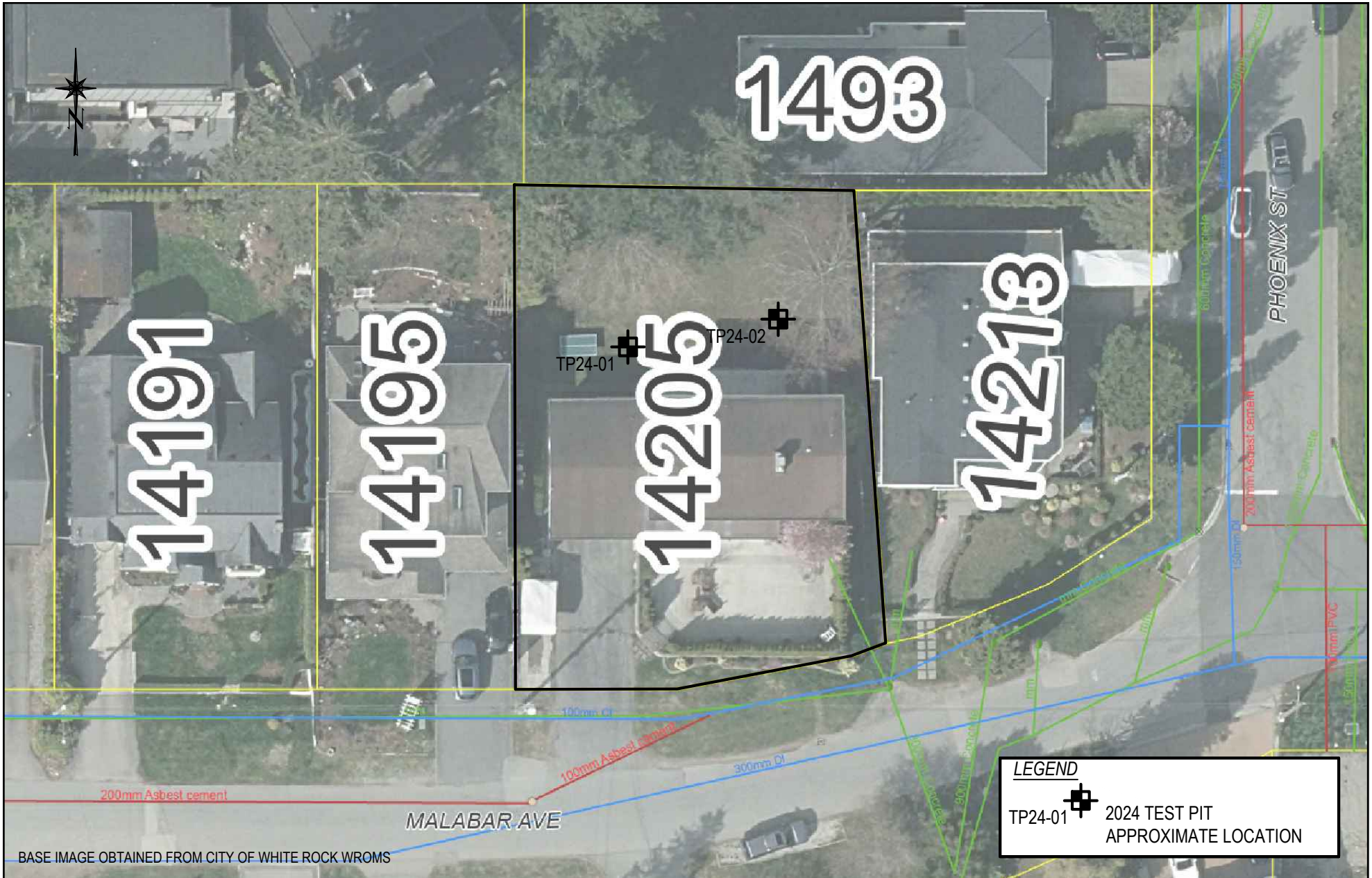
8. SUBCONSULTANTS AND CONTRACTORS

Engineering studies frequently require hiring the services of individuals and companies with special expertise and/or services which Braun does not provide. These services are arranged as a convenience to our Clients, for the Client's benefit. Accordingly, the Client agrees to hold the Company harmless and to indemnify and defend Braun from and against all claims arising through such Subconsultants or Contractors as though the Client had retained those services directly. This includes responsibility for payment of services rendered and the pursuit of damages for errors, omissions or negligence by those parties in carrying out their work. These conditions apply to specialized subconsultants and the use of drilling, excavation and laboratory testing services, and any other Subconsultant or Contractor.

9. SITE SAFETY

Braun assumes responsibility for site safety solely for the activities of our employees on the jobsite. The Client or any Contractors on the site will be responsible for their own personnel. The Client or his representatives, Contractors or others retain control of the site. It is the Client's or the Client's Contractors responsibility to inform Braun of conditions pertaining to the safety and security of the site – hazardous or otherwise – of which the Client or Contractor is aware.

Exploration or construction activities could uncover previously unknown hazardous conditions, materials, or substances that may result in the necessity to undertake emergency procedures to protect workers, the public or the environment. Additional work may be required that is outside of any previously established budget(s). The Client agrees to reimburse Braun for fees and expenses resulting from such discoveries. The Client acknowledges that some discoveries require that certain regulatory bodies be informed. The Client agrees that notification to such bodies by Braun will not be a cause for either action or dispute.



BASE IMAGE OBTAINED FROM CITY OF WHITE ROCK WROMS

LEGEND

TP24-01 2024 TEST PIT APPROXIMATE LOCATION



Client	Paragon Homes Ltd.				Title	LOCATION PLAN		
Project	Proposed Subdivision 14205 Malabar Avenue, White Rock, BC							
Project no.	24-9848	Drawn	Design	Checked	Date	Scale	Drawing no.	
		DD	AC	JO	May 14, 2024	1:400	24-9848-01	

Test Pit Log: TP24-01

File: 24-9848
 Project: Proposed Subdivision
 Client: Paragon Homes Ltd.
 Location: 14205 Malabar Avenue, White Rock, BC



Depth	Thickness (mm)	Sample	Soil Description	Sample #	Water cont.	Remarks
0	0		dark-brown, damp, firm, sandy SILT, some organics, occasional roots/rootlets (FILL/ORGANICS)			
1			rust-brown, damp, compact SAND, some silt, trace gravel, occasional roots/rootlets			
2			brown, damp, dense to very dense SAND, some silt to silty SAND, trace to some gravel			
3		○		S1	15%	
4		○		S2	16%	
5		○		S3	20%	
6		○		S4	19%	
6			End of Test Hole @ 1.8m			
7						
8						
9						
10						

Equipment: Tracked Excavator	Datum: Ground Surface	Logged By: AC
Sampling Method: Lump Sample	Water Depth: Not Encountered (during excavation)	Exploration Date: May 18, 2024
		Dwg No.: 24-9848-TH01
		Page: 1 of 1

Test Pit Log: TP24-02

File: 24-9848
 Project: Proposed Subdivision
 Client: Paragon Homes Ltd.
 Location: 14205 Malabar Avenue, White Rock, BC



PTP# 1002594

Depth	Thickness (mm)	Sample	Soil Description	Sample #	Water cont.	Remarks
0	0		GRASS OVER			
0		○	dark-brown, damp, loose, silty SAND, some organics, occasional roots/rootlets (FILL/ORGANICS)	S1	26%	
1		○	rust-brown, damp, compact SAND, some silt, trace gravel, occasional roots/rootlets	S2	27%	
2		○	brown, occasionally rust-mottled, damp, stiff, fine sandy SILT	S3	28%	
3		○	brown, damp, dense to very dense SAND, some silt to silty SAND, trace to some gravel	S4	13%	
4		○		S5	16%	
5		○		S6	15%	
			End of Test Hole @ 1.5m			

Equipment: Tracked Excavator	Datum: Ground Surface	Logged By: AC
Sampling Method: Lump Sample	Water Depth: Not Encountered (during excavation)	Exploration Date: May 18, 2024
		Dwg No.: 24-9848-TH02
		Page: 1 of 1